



COMPANY

PROJECT

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Beam1- US.wwbu

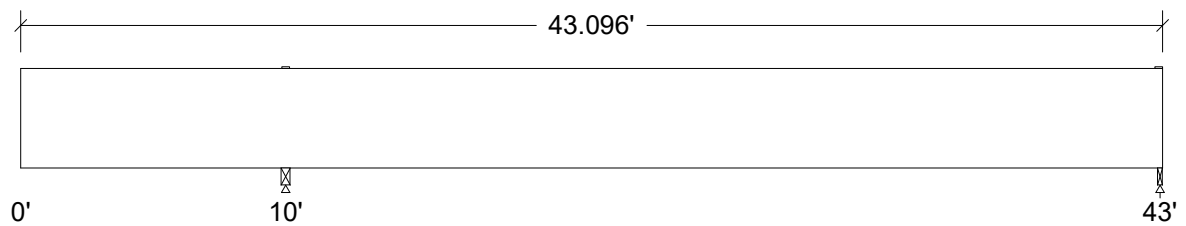
Design Check Calculation Sheet

WoodWorks Sizer 13.2.1

Loads:

Load	Type	Distribution	Pat- tern	Location [ft] Start End	Magnitude Start End	Unit
D	Dead	Full UDL	No		410.0	plf
S	Snow	Full UDL	Yes		680.0	plf
Self-weight	Dead	Full UDL	No		111.4	plf

Maximum Reactions (lbs), Bearing Capacities (lbs) and Bearing Lengths (in) :



Unfactored:					
Dead			14606		7852
Snow			19050		10770
Factored:					
Total			27942		15410
Bearing:					
Capacity					
Beam			31680		16027
Support			27942		15410
Des ratio					
Beam			0.88		0.96
Support			1.00		1.00
Load comb			#2		#4
Length			4.16		2.29
Min req'd			4.16**		2.29**
Cb			1.09		1.00
Cb min			1.09		1.00
Cb support			1.00		1.00
Fcp sup			625		625

**Minimum bearing length governed by the required width of the supporting member.

Glulam-Balanced, West Species, 24F-1.8E WS, 10-3/4"x45"

Supports: All - Timber-soft Beam, D.Fir-L No.2

Total length: 43.1'; Clear span: 9.827', 32.731'; Volume = 144.8 cu.ft.; 30 laminations, 10-3/4" maximum width,

Lateral support: top = at supports, bottom = at supports;

This section PASSES the design code check.

Analysis vs. Allowable Stress and Deflection using NDS 2024 :

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	$f_v = 44$	$F_v' = 305$	psi	$f_v/F_v' = 0.14$
Bending(+)	$f_b = 388$	$F_b' = 2161$	psi	$f_b/F_b' = 0.18$
Bending(-)	$f_b = 165$	$F_b' = 2355$	psi	$f_b/F_b' = 0.07$
Deflection:				
Interior Live	$0.08 = < L/999$	$1.65 = L/240$	in	0.05
Total	$0.19 = < L/999$	$2.20 = L/180$	in	0.09
Cantil. Live	$-0.06 = < L/999$	$1.00 = L/120$	in	0.06
Total	$-0.14 = L/853$	$1.33 = L/90$	in	0.11

Additional Data:

FACTORS:	F/E(psi)	CD	CM	Ct	CL	CV	Cfu	Cr	Cfrt	Notes	Cvr	LC#
F_v'	265	1.15	1.00	1.00	-	-	-	-	1.00	1.00	1.00	2
$F_b'+$	2400	1.15	1.00	1.00	0.906	0.783	-	-	1.00	1.00	-	4
$F_b'-$	2400	1.15	1.00	1.00	0.985	0.853	-	-	1.00	1.00	-	2
F_{cp}'	650	-	1.00	1.00	-	-	-	-	1.00	-	-	-
E'	1.8 million	1.00	1.00	-	-	-	-	-	1.00	-	-	4
E_{min}'	0.85 million	1.00	1.00	-	-	-	-	-	1.00	-	-	4

Only the lesser of CL and CV is applied, as per NDS 5.3.6

CRITICAL LOAD COMBINATIONS:

Shear : LC #2 = D + 0.7S
 Bending(+): LC #4 = D + S (pattern: sS)
 Bending(-): LC #2 = D + 0.7S
 Deflection: LC #4 = (live)
 LC #4 = (total)
 Bearing : Support 1 - LC #2 = D + 0.7S
 Support 2 - LC #4 = D + S (pattern: sS)
 Load Types: D=dead S=snow
 Load Patterns: s=S/2, X=L+S or L+Lr, _=no pattern load in this span
 Load combinations: ASD Basic from ASCE 7-22 2.4; all LC's listed in the Analysis report

CALCULATIONS:

$V_{max} = 17968$, $V_{design} = 14063$ (NDS 3.4.3.1(a)) lbs
 $M(+) = 117430$ lbs-ft; $M(-) = 49869$ lbs-ft
 $EI = 146936e06$ lb-in²
 "Live" deflection is due to all non-dead loads (live, wind, snow...)
 Total deflection = 1.50 permanent + "live"
 Lateral stability(+): $L_u = 33.00'$ $L_e = 58.75'$ $RB = 16.6$; L_u based on full span
 Lateral stability(-): $L_u = 10.00'$ $L_e = 18.69'$ $RB = 9.3$; L_u based on full span

Design Notes:

1. Analysis and design are in accordance with the ICC International Building Code (IBC 2024) and the National Design Specification (NDS 2024), using Allowable Stress Design (ASD). Design values are from the NDS Supplement.
2. Please verify that the default deflection limits are appropriate for your application.
3. Glulam design values are for materials conforming to ANSI 117-2015 and manufactured in accordance with ANSI A190.1-2012
4. Grades with equal bending capacity in the top and bottom edges of the beam cross-section are recommended for continuous beams.
5. GLULAM: bxd = actual breadth x actual depth.
6. Glulam Beams shall be laterally supported according to the provisions of NDS Clause 3.3.3.
7. GLULAM: bearing length based on smaller of $F_{cp}(\text{tension})$, $F_{cp}(\text{comp'n})$.